

# UHAINA : A parallel high performance unstructured near-shore wave model

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## Abstract:

UHAINA is a new phase-resolving free surface wave model for coastal engineering problems. It is based on the most advanced and recent contributions in coastal modelling from the french institutes EPOC, IMAG, IMB, and INRIA BSO. It solves a non-classical version of the depth-integrated fully-nonlinear and weakly-dispersive equations of Green-Naghdi, which allows an efficient numerical implementation. UHAINA relies on libraries developed at the INRIA BSO center, such as AeroSol for its hydrodynamic core, and PaMPA and SCOTCH to handle data management for distributed memory parallel computation. The use of these libraries, in particular AeroSol, offers a wide range of possibilities including arbitrary high-order finite element discretizations, hybrid meshes (structured and unstructured), as well as an advanced programming environment specially designed by the purpose of performance and HPC. These properties will lead in the coming years to the release of a new efficient and robust open source wave modelling platform, available for a large community of users and very suitable for practical coastal applications.

**Keywords:** Green Naghdi equations, Phase-resolving wave model, Wave breaking, Discontinuous Finite Element, Unstructured meshes.

## 1. Introduction

For several decades the population density in coastal areas has significantly increased. This trend is expected to continue in the next decades. Consequently, extreme ocean

41 events, such as tsunamis or storm waves, have increasing damaging consequences. This  
42 makes risk assessment a crucial element for the safe development of these communities.  
43 In this context, it is essential to dispose of robust and yet efficient models for predicting  
44 extreme events involving the propagation of waves in the near shore, and their impact  
45 on the coast. Over the past years, strong efforts have been made by the French  
46 laboratories and research centers EPOC, IMAG, IMB, and INRIA BSO to advance the  
47 state of wave modelling and flooding simulations for coastal engineering applications.  
48 These efforts have recently focused on developing and validating the newly developed  
49 non-hydrostatic wave-flow model UHAINA. This model will integrate the collective  
50 know how on non-linear wave modelling, high order numerical discretizations, and high  
51 performance object oriented implementation developed in the last decade. Typical  
52 applications of UHAINA will be the study of the propagation and transformation of  
53 waves in the surf and swash zones, such as wave shoaling, dispersion and breaking  
54 together with coastal flooding and structure overtopping.

55 In recent years, Boussinesq wave models have become a useful tool for modeling  
56 surface wave transformation from deep water to the swash zone. Great improvements  
57 have been obtained in the derivation and mathematical understanding of particular  
58 asymptotic models able to describe the behaviour of the solution in some physical  
59 specific regimes; a recent review on different existing models is given in LANNES &  
60 BONNETON (2009). In particular, great efforts have been focused on improving the  
61 range of model applicability with respect to classical restrictions to both weak  
62 dispersion and weak nonlinearity. The use of the so-called *fully-nonlinear* formulation  
63 of GREEN & NAGHDI (1978) eliminates the restriction to weak nonlinearity,  
64 enhancing the models capabilities in the surf and swash zones, where the wave breaking  
65 point is attended in conditions of increased nonlinearity. For these reasons, the Green-  
66 Naghdi equations have gained a lot of attention in the recent past. UHAINA uses a non-  
67 classical variant of this model with improved linear dispersion properties and which  
68 allows a faster solution procedure (LANNES & MARCHE, 2015).

69 Phase-resolved modelling, based on Boussinesq-type equations and in conjunction with  
70 suitable numerical techniques, has emerged as a mature discipline and generated the  
71 most widely employed predictive tools in coastal engineering and in morphodynamics,  
72 as *e.g.* FUNWAVE (KIRBY *et al.*, 1998) or COULWAVE (LYNETT & LIU, 2002).  
73 UHAINA fits into this category of models, embedding the most recent research  
74 progresses in coastal wave modelling, making them accessible to anyone. To do so, the  
75 platform relies on libraries developed at Inria, such as the AeroSol finite element library  
76 (for its hydrodynamic core), and the libraries PaMPA and SCOTCH to handle  
77 transparently for the model developer data management in distributed memory parallel  
78 computations. The use of these libraries, and in particular AeroSol, offers a wide range  
79 of possibilities including both continuous and discontinuous High-Order finite element

80 discretizations, hybrid meshes (structured and unstructured), as well as a code design  
81 driven by performance purposes (HPC) using advanced C++ programming techniques.  
82 In what follows, Section 2 summarises the essential model, as well as numerical and  
83 computational aspects featuring in UHAINA. In Section 3, a few application examples  
84 are then presented, reflecting both capabilities and performances. Finally, Section 4  
85 concludes the paper with some closing remarks.

86

## 87 **2. Modelling framework**

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### 89 2.1 Physical model

90 Modeling nonlinear coastal wave processes, such as inundation, wave runup, bore  
91 propagation, tsunami propagation, and infragravity waves, requires efficient and  
92 accurate computing of the evolution of highly nonlinear and dispersive surface wave  
93 fields in complex coastal environments. UHAINA relies on a phase-resolving approach,  
94 based on the fully-nonlinear and weakly-dispersive Boussinesq wave model of GREEN  
95 & NAGHDI (1978). This system of equations shares the same linear dispersion  
96 properties of the original Boussinesq model of PEREGRINE (1967), however an  
97 equivalent model with improved dispersion properties has been proposed in (HAZEL  
98 *et al.*, 2011), by the introduction of a tuning parameter  $\alpha$  multiplying some dispersive  
99 terms. The new formulation of the governing equations, proposed by BONNETON *et*  
100 *al.* (2011), allows the system of governing equations to be rewritten as follows:

$$101 \quad \partial_t \zeta + \nabla \cdot (h\mathbf{u}) = 0 \quad (1)$$

$$102 \quad \partial_t (h\mathbf{u}) + \nabla \cdot (h\mathbf{u} \otimes \mathbf{u}) + gh\nabla\zeta = \phi \quad (2)$$

$$103 \quad (I + \alpha T)(\phi) - T(gh\nabla\zeta) + hQ(\mathbf{u}) = 0 \quad (3)$$

104 having used  $\zeta$  to indicate the free surface elevation,  $h$  for the total water depth,  $\mathbf{u}$  for the  
105 velocity vector, and where  $\phi$  accounts for the non-hydrostatic effects,  $I$  is the identity  
106 matrix, while  $T$  and  $Q$  are operators containing high-order derivatives in space (for more  
107 details on their definitions please refer to the cited works). A two steps solution  
108 procedure is applied to the system (1)-(3), as described in FILIPPINI *et al.* (2016). It  
109 consists in: an elliptic phase (3) in which the source term  $\phi$  is computed by inverting the  
110 coercive operator associated to the dispersive effects; an hyperbolic phase in which the  
111 flow variables are evolved by solving the Shallow Water equations (1)-(2), with all non-  
112 hydrostatic effects accounted for by the source  $\phi$  computed in the elliptic phase. The  
113 main advantage of this formulation is the presence of the operator  $(I+\alpha T)$ , which makes  
114 the model robust with respect to high frequency perturbations, an interesting property  
115 for numerical computations. However, the inversion of the  $(I+\alpha T)$  matrix, for the  
116 solution of the elliptic phase, is the most computationally demanding part of the whole  
117 solution process. This is due to the fact that, firstly,  $(I+\alpha T)$  is a matricial second order  
118 differential operator acting on two-dimensional vectors and this structure entails a  
119 coupling of the time evolutions of the two components of  $h\mathbf{u}$  through (2). Secondly,

120  $T(h(\mathbf{x}, t))$  is a time-dependent operator, through the dependence on  $h$ : the corresponding  
121 matrices have, thus, to be assembled at each time step or sub-steps. In order to  
122 overcome these drawbacks without loosing the benefits of the formulation (1)-(3),  
123 UHAINA exploits new very promising non-classical models derived by LANNES &  
124 MARCHE (2015). While keeping the same asymptotic  $O(\mu^2)$  order (being  $\mu = (h_0 / \lambda)^2$   
125 the dispersion parameter, with  $h_0$  the reference water depth and  $\lambda$  the typical wave  
126 length), and linear properties of the original model, LANNES & MARCHE (2015) have  
127 shown that it is possible to rewrite the elliptic equation (3) in a way such that the new  
128 operator to invert to be either block diagonal or block diagonal and time-independent,  
129 leading to considerable improvement in terms of computational time, since it allows to  
130 perform the corresponding matrix assembling and factorization in a pre-processing step.  
131 For the representations of dissipative wave-breaking events, UHAINA exploits the  
132 hybrid strategy of TISSIER *et al.* (2012), KAZOLEA *et al.* (2014) and DURAN &  
133 MARCHE (2017), by locally reverting to the Shallow Water equations to model energy  
134 dissipation in breaking regions.

135

## 136 2.2 Numerical discretization

137 From the numerical point of view the Green-Naghdi equations have been discretized  
138 using different numerical techniques including Finite Differences (FD), Finite Elements  
139 (FE) and Finite Volumes (FV) approaches. The major challenges that need to be dealt  
140 with are the approximation of the complex higher order derivative terms, in respect of  
141 the accuracy requirements on the schemes in terms of low dispersion errors. Fully  
142 unstructured solvers, allowing for adaptive mesh refinement, have been proposed based  
143 either on a hybrid FV/FE approach (FILIPPINI *et al.*, 2016) or on a discontinuous FE  
144 approach (DURAN & MARCHE, 2017). Inspired by these works, UHAINA is focused  
145 on the application of a FE discretization of the governing equations. This gives a  
146 framework to naturally introduce higher order polynomial representation of the  
147 unknowns and of their derivatives.

148 UHAINA adopts an arbitrary high-order discontinuous FE discretization of the  
149 hyperbolic phase, exploiting the robustness and shock capturing capabilities of this  
150 approach in wave breaking regions, where the Shallow Water equations are solved, due  
151 to the hybrid breaking model used. The use of a nodal approach together with the pre-  
152 balanced formulation of the hyperbolic part of the model allows to combine two  
153 important properties (DURAN & MARCHE, 2017): firstly, an efficient quadrature free  
154 treatment for the integrals which are not involved into the equilibrium state  
155 preservation; secondly, a quadrature-based treatment with a lower computational cost,  
156 needed to exactly compute the surface and face integrals involved in the preservation of  
157 the steady states at rest. Moreover discontinuous FE approach has a compact stencil; a  
158 property which makes it well-suited for parallel computing. Concerning the  
159 discretization of the elliptic phase, a second order FE approach is here used, leading for

160 the fully coupled scheme to a phase accuracy very close to that of a fourth order FD  
161 method, as stated in the works of FILIPPINI *et al.* (2016) and DURAN & MARCHE  
162 (2017).

163

### 164 2.3 HPC Implementation

165 High-order accuracy, computational efficiency and parallelism are among the main  
166 targets of the UHAINA platform. In order to achieve these objectives UHAINA relies  
167 on the Aerosol library, developed at the INRIA SO institute. Aerosol is a C++ library,  
168 devoted to the solution of complex CFD problems and recently adapted to deal also  
169 with hydrodynamic applications. It is a high order finite element library based on both  
170 continuous and discontinuous elements on hybrid meshes, involving triangles and  
171 quadrangles in two dimensions. More precisely, it enables the generation of finite  
172 element classes up to the fourth order polynomial approximation. The code design is  
173 driven by the purpose of performance using advance C++ programming techniques and  
174 employs an efficient parallel implementation, which allows high performance  
175 computing on massively parallel architectures. Aerosol depends on the PaMPA library  
176 for memory handling, for mesh partitioning, and for abstracting the MPI layer, and it is  
177 also linked with external linear solvers (e.g. BLAS, PETSc 4 and MUMPS 5 ).

178

179 At present, only the hyperbolic part of the model is operational, while the development  
180 of the non-hydrostatic part of the model is currently underway.

181

## 182 **3. Preliminary results**

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### 184 3.1 Convergence test

185 Firstly, we report a convergence study performed on an academic test, which allows us  
186 to asses the theoretical order of accuracy of the implemented scheme.

187 The test proposed consists in propagating a subcritical flow over a submerged bump  
188 described by the function  $z = 0.5(2\pi)^{-1/2}\exp(-0.5(x - 10)^2)$ . The free surface  
189 elevation is initially set as constant at  $\eta = 2$  [m] and a constant discharge of  $q = 4.42$   
190 [m<sup>2</sup>/s] is injected from the left boundary of the domain, while an open boundary is  
191 simulated on the right. The result of the simulation is shown in Figure 1 (left). The test  
192 is performed using the nonlinear shallow water model, for which an analytical solution  
193 to this problem exists (GOUTAL & MAUREL, 1998). This allows us to perform a grid  
194 convergence of the error using different order of polynomial approximation in our  
195 numerical scheme. The test case is performed on a set of four meshes successively  
196 dividing the space step by two up to  $dx = 0.125$  [m], while keeping the time step small  
197 enough to ensure that the leading error order is provided by the spatial discretization.  
198 The slopes obtained from the errors, in Figure 1 (right), reveal that the convergence  
199 rates of the scheme match the theoretical values for all the combinations.

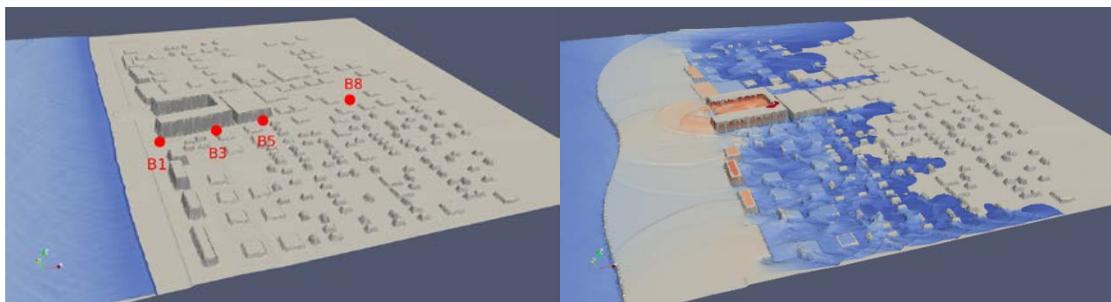
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*Figure 1. Left: Steady subcritical flow over a bump; illustration of the profile of water surface and bottom. Right : convergence of the  $L^2$  norm of the error with respect to the inverse of the number of degrees of freedom, when polynomial approximations of order  $P^0$ ,  $P^1$ ,  $P^2$  and  $P^3$  are used in the scheme.*

### 3.1 Real case application

Hereafter, UHAINA is applied to numerically reproduce the laboratory experiment of PARK *et al.* (2013). This is a very recent benchmark test for tsunami inundation of an urban waterfront. A 1:50 scale idealization of the town Seaside, Oregon was designed to observe the impact of a tsunami wave and measure the water flows which are produced around the city buildings. The scale model was installed in a rectangular basin with a wavemaker on the offshore boundary. Free surface elevation and velocity time series were measured and analyzed at 31 points along 4 transects (please refer to the cited work for the precise setup of the experiment and gauges positions).

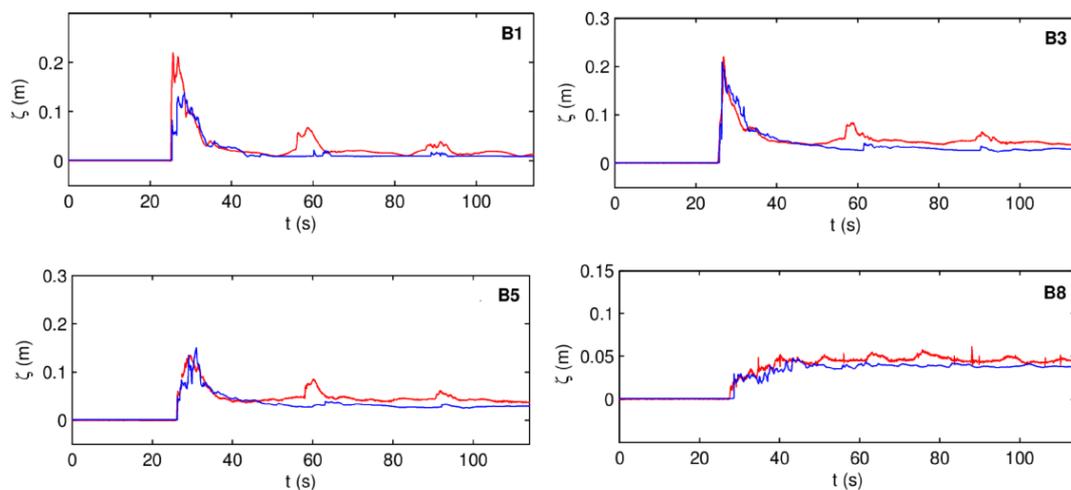
The numerical simulation has been performed using the shallow water model. Figure 2 shows two different views of the computational domain during the simulation, one before (left) and the other just after (right) the tsunami impacting the town.



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*Figure 2. Three dimensional views of the Seaside numerical simulation, at two different instant of the computation: before (left) and just after (right) the tsunami arrival. Red bullets correspond to some gauges positions.*

224 The computed free surface time series at some significant gauges (Figure 2, left),  
 225 located along the central street of the city, are illustrated in Figure 3 and compared with  
 226 respect to the experimental ones. The comparisons show a good agreement between the  
 227 numerical and the physical model in the most of the locations inspected. However, as  
 228 known, the shallow water equations, applied for the numerical simulation,  
 229 underestimate the shoaling of the tsunami wave over the sloping beach. This cause the  
 230 computed peak of the first incoming wave to be smaller than the experimental one, an  
 231 effect which is visible in the signal registered at gauge B1 (the one situated close to the  
 232 seaside).



233  
 234 *Figure 3. Time series of the free surface elevation at some gauges positions along the*  
 235 *main street of the city, perpendicular to the sea line: blue lines indicate the results of*  
 236 *the simulation, while red lines stay for the experimental ones.*

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#### 238 **4. Conclusions and perspectives**

239 A new non-hydrostatic wave-flow modelling platform named UHAINA is presented in  
 240 this work. It is devoted to the prediction of surface waves transformation processes in  
 241 coastal waters, gathering the following numerical properties: it provides arbitrarily high-  
 242 order discretization of a new non-classical formulation of the Green-Naghdi equations;  
 243 it works on unstructured meshes; it exploits an efficient parallel implementation,  
 244 allowing HPC, through the use of the Aerosol library and its dependencies.

245 To validate the currently operational part of the code and to demonstrate its potential,  
 246 two test cases have been presented for demonstration and validation purposes, showing  
 247 that an arbitrary order of accuracy of the numerical scheme is correctly obtained and  
 248 that the scheme is able to correctly reproduce a realistic case of study in a complex  
 249 flooding scenario.

250 Further developments will lead in the coming years to a favorable environment for a  
 251 large community of users to perform real-time large simulations with pre- and post-

252 processing of the data, and will make UHAINA a new generation robust and  
253 computationally efficient phase-resolving numerical wave model very suitable for  
254 practical application studies.

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## 256 **5. References**

257 BONNETON P., CHAZEL F., LANNES D., MARCHE F., TISSIER M. (2011). A  
258 *splitting approach for the fully nonlinear and weakly dispersive Green-Naghdi model*. J.  
259 Comp. Phys., Vol. 230(4), pp 1479-1498.

260 CHAZEL F., LANNES D., MARCHE F. (2011). *Numerical simulation of strongly*  
261 *nonlinear and dispersive waves using a Green–Naghdi model*. J. Sc. Comp., Vol. 48(3).

262 GOUTAL N., MAUREL F. (1998) *Dam-break wave simulation*. Proceeding in 1st  
263 CADAM workshop

264 DURAN A., MARCHE F. (2017). *A discontinuous Galerkin method for a new class of*  
265 *Green–Naghdi equations on simplicial unstructured meshes*. Appl. Mat. Mod., Vol. 45.

266 FILIPPINI A.G., KAZOLEA M., RICCHIUTO M. (2016) *A flexible genuinely*  
267 *nonlinear approach for nonlinear wave propagation, breaking and run-up*. J. Comp.  
268 Phys., Vol. 310, pp 381-417.

269 GREEN A.E., NAGHDI P.M. (1978). *A derivation of equations for wave propagation*  
270 *in water of variable depth*. J. Fluid Mech., Vol. 78, pp 237-246.

271 KAZOLEA M., DELIS A.I., SYNOLAKIS C. (2014). *Numerical treatment of wave*  
272 *breaking on unstructured finite volume approximations for extended Boussinesq type*  
273 *equations*. J. Comp. Phys., Vol. 271, pp 281-305.

274 KIRBY J.T., WEI G., CHEN Q., KENNEDY A., DALRYMPLE R.A. (1998)  
275 *FUNWAVE 1.0 : Fully nonlinear Boussinesq wave model documentation and User’s*  
276 *manual*. CACR-98-06, University of Delaware, Newark, DE 19716, USA.

277 LANNES D., BONNETON P. (2009). *Derivation of asymptotic two-dimensional time-*  
278 *dependent equations for surface water wave propagation*. Phys. of fluids, Vol. 21.

279 LANNES D., MARCHE F. (2015). *A new class of fully nonlinear and weakly dispersive*  
280 *Green-Naghdi models for efficient 2D simulations*, J. Comp. Phys., Vol. 282.

281 LYNETT P.J., LIU P.L. (2002) *Modeling wave generation, evolution, and interaction*  
282 *with depth-integrated, dispersive wave equations*. *COULWAVE code manual* Cornell  
283 University, Ithaca, NY, USA.

284 PARK H., COX D.T., LYNETT P.J., WIEBE D.M., SHIN S. (2013) *Tsunami*  
285 *inundation modeling in constructed environments: A physical and numerical*  
286 *comparison of free-surface elevation, velocity, and momentum flux*. Coast. Eng., Vol.  
287 79, pp 9-21.

288 PEREGRINE D.H. (1967). *Long waves on a beach*. J. Fluid Mech., Vol. 27.

289 TISSIER M., BONNETON P., MARCHE F., CHAZEL F., LANNES D. (2012). *A new*  
290 *approach to handle wave breaking in fully nonlinear Boussinesq models*. Coast. Eng.,  
291 Vol. 67, pp 54-66.